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**«ПРОБЛЕМИ НАДІЙНОСТІ ТА ДОВГОВІЧНОСТІ  
ІНЖЕНЕРНИХ СПОРУД І БУДІВЕЛЬ  
НА ЗАЛІЗНИЧНОМУ ТРАНСПОРТІ»**



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**Збірник містить тези доповідей науковців вищих навчальних закладів України та інших країн, підприємств транспортної та будівельної галузі за трьома напрямками: залізниця, автомобільні дороги, промисловий транспорт і геодезичне забезпечення; будівельні конструкції, будівлі та споруди; будівельні матеріали, захист і ремонт конструкцій та споруд.**

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**The proceedings include abstracts of presentations by researchers from higher education institutions in Ukraine and other countries, as well as representatives of enterprises in the transport and construction industries. The topics are organized into three main areas: railways, highways, industrial transport, and geodetic support; building structures, buildings, and facilities; and construction materials, including the protection and repair of structures and facilities.**

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**МЕТОД ПРОГНОЗУВАННЯ РОЗШИРЕННЯ ТРИЩИН ДЛЯ БЕТОННИХ  
ТА ЗАЛІЗОБЕТОННИХ ЕЛЕМЕНТІВ****CRACKS SPREADING PROGNOSTICATION METHOD FOR CONCRETE  
AND REINFORCED CONCRETE ELEMENTS**

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The rupture cracks development, brittle failure and design for prevention the brittle failure of concrete elements is considered [1-3]. Under the brittle failure of being tested elements the undulating relationship “Load  $F$  – crack length  $l$ ” is observed. This curve  $F=f(l)$  includes initial ascending and subsequent disascending branches, separated with the maximum point of critical crack length  $l_{cr}$  and critical (maximal, ultimate) load  $F_{cr}$ , which determine the strength under brittle failure. Thus, the adequate model of brittle failure ought to determine the relationship  $F=f(l)$  together with it’s the critical values  $l_{cr}$  and  $F_{cr}$ , which solve the practical problem of prevention the brittle failure. Nevertheless, the receipt of accurate enough relationship  $F=f(l)$  is impossible or highly difficult through the known Fracture Mechanics (FM) models. Therefore, for overcoming of the known FM model’s demerits the specific FM model is offered, which is based on modelling the stress distribution only along the crack Fracture Process Zone (FPZ [4]) with elastic concrete behavior out of the FPZ. Thus, peculiar stress distribution was called “Physically Verisimilar Stress (PVS)” and the latter is accepted as basis of the being offered PVS model, which uses three material parameters: maximal stress  $\sigma_m$ , intra-structural linear size “ $a$ ” and dimensionless value “ $n$ ”, taking into account the plastic properties of material. The PVS model concrete parameters, obtained in the authors tests, are suggested.

Strength problems are solved by the “Modified Method of the Sections (MMS)”, in which the PVS is applied along the FPZ unlike the known Method of Sections with the asymptotic stresses in the FPZ. The uses system of equations reflects in detail the equilibrium of external and internal forces of elements with crack and the one is sufficient for different problems solving without strains consideration, that simplifies essentially designs. As a result, the offered design method is “static method of

ultimate equilibrium” for brittle failure, in which the strength criterion is just the PVS.

The PVS model and the MMS lead to acceptable in practice method of strength design, which is considerably simpler than widespread in FM of concrete the Phase Field Method, demanding the highly complicated computer programs, based on the complex Finite Element Method, while the MMS can be realized by means of the Table Processor MS Excel programs and by other easy models.

The offered PVS model and the MMS allow to predict the crack development as its initial stable growing up to the critical (ultimate) values of the crack length  $l_{cr}$  and load  $F_{cr}$  as subsequent unstable crack spreading [5].

Development of the “crack-FPZ combination” was considered on the basis of thermodynamic notions, laws and methods as the new phases origin in the concrete structure. The thermodynamic approach led to the relationships, confirming the PVS model.

The examples of designs are stated and sufficient nearness of the theoretic strength to experimental one is shown. Herewith the infinite tensile plate with crack (Griffith`s problem), long tensile strips with central and with one-sided cracks, concrete and reinforced concrete bending elements with one-sided cracks are considered.

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